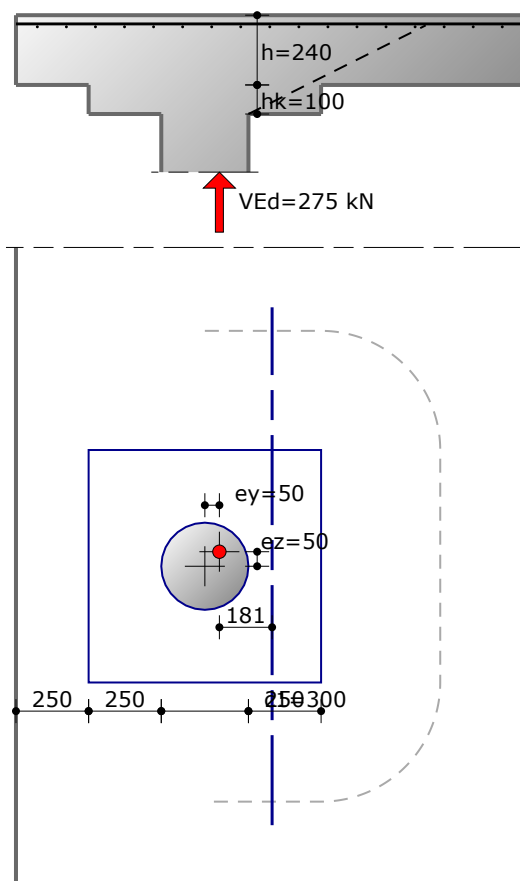


GENERAL

File :Struct4U\website\voorbeeld uitdraai\XConstruct\ VoorbeeldenDutch.xcst

Consequence class : CC2

PUNCHING: Punch**INPUT DATA**

Concrete grade	C30/37
Steel grade	B500B
Longitudinal reinf. y	10-100
Longitudinal reinf. z	10-100
First layer	Longitudinal reinf. y
Cover c	25 mm
Angle shear reinforcement	90 degrees
V_{Ed}	275 kN
Eccentricity	$e_y = 50$ mm
q_{Ed}	0 kN/m ²

 $e_z = 50$ mm

CALCULATION according to Eurocode 2

Applied standards: : NEN-EN 1992-1-1+C1:2011/NB:2016+A1:2020 nl

6.4.3 Punching shear calculation

$$d_{\text{eff}} = \frac{d_y + d_z}{2} = \frac{210 + 200}{2} = 205 \text{ mm} \quad \dots(6.32)$$

$$v = 0,6 \left[1 - \frac{f_{ck}}{250} \right] = 0,6 \times \left[1 - \frac{30}{250} \right] = 0,528 \quad \dots(6.6N)$$

$$v_{Rd,max} = 0,4 v f_{cd} = 0,4 \times 0,528 \times 20 = 4,22 \text{ MPa}$$

$$c_1 / (2 c_2) = 800 / (2 \times 800) = 0,5 \quad k = 0,45 \quad \dots(T 6.1)$$

$$W_1 = 2,365 \text{ m}^2 \quad \dots(6.40)$$

$$\beta = \frac{u_1}{u_{1^*}} + k \frac{u_1}{W_1} e_{\text{par}} = \frac{3845}{2888} + 0,45 \times \frac{3845}{2,365 \times 10^6} \times 50 = 1,37 \quad \dots(6.44)$$

$$u_0 = \pi D = \pi \times 300 = 942 \text{ mm}$$

$$v_{Ed} = \beta \frac{V_{Ed}}{u_0 d} = 1,37 \times \frac{275 \times 10^3}{942 \times 305} = 1,31 \text{ MPa} < v_{Rd,max} = 4,22 \text{ MPa} \quad \dots(6.53)$$

$$k = 1 + \sqrt{\frac{200}{d}} = 1 + \sqrt{\frac{200}{205}} = 1,988 \leq 2,0$$

$$\rho_{ly} = A_{sly} / (b_w d_y) = 785 / (1000 \times 210) = 0,00374$$

$$\rho_{lz} = A_{slz} / (b_w d_z) = 785 / (1000 \times 200) = 0,00393$$

$$\rho_l = \sqrt{\rho_{ly} \rho_{lz}} = \sqrt{0,004 \times 0,004} = 0,0037 < 0,02$$

$$v_{Rd,c1} = C_{Rd,c} k (100 \rho_l f_{ck})^{1/3} + k_1 \sigma_{cp} = \quad \dots(6.47)$$

$$= 0,12 \times 1,988 \times (100 \times 0,004 \times 30)^{1/3} + 0,15 \times 0 = 0,54 \text{ MPa}$$

$$v_{\text{min}} = 0,035 k^{3/2} f_{ck}^{1/2} = 0,035 \times 1,988^{3/2} \times 30^{1/2} = 0,54 \text{ MPa} \quad \dots(6.3N)$$

$$v_{Rd,c2} = v_{\text{min}} + k_1 \sigma_{cp} = 0,537 + 0,15 \times 0 = 0,54 \text{ MPa}$$

$$u_1 = 3845 \text{ mm} \quad u_{1^*} = 2888 \text{ mm} \quad A_1 = 2,08 \text{ m}^2$$

$$V_{Ed} = V_{Ed} - A_1 q_{Ed} = 275 - 2,08 \times 0,00 = 275 \text{ kN}$$

$$v_{Ed} = \beta \frac{V_{Ed}}{u_1 d} = 1,37 \times \frac{275 \times 10^3}{3845 \times 205} = 0,48 \text{ MPa} < v_{Rd,c} = 0,54 \text{ MPa}$$

Punching shear reinforcement is not necessary